

Third Party Audit of ROB's and FOB's Of SCR



INSPECTION OF ROB NO. 561A AT CHITTOOR RAILWAY STATION

Final Report
SEPTEMBER | 2020

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1. VISUAL INSPECTION

Name of the Project : VISUAL INSPECTION OF ROB 561A
Location : NEAR CHITTOOR RAILWAY STATION (32/3-4)
Location Co-ordinates : 13°13'00.3"N 79°06'12.3"E
Date of Inspection : 27th NOVEMBER, 2019
Date of NDT : 03rd September, 2020



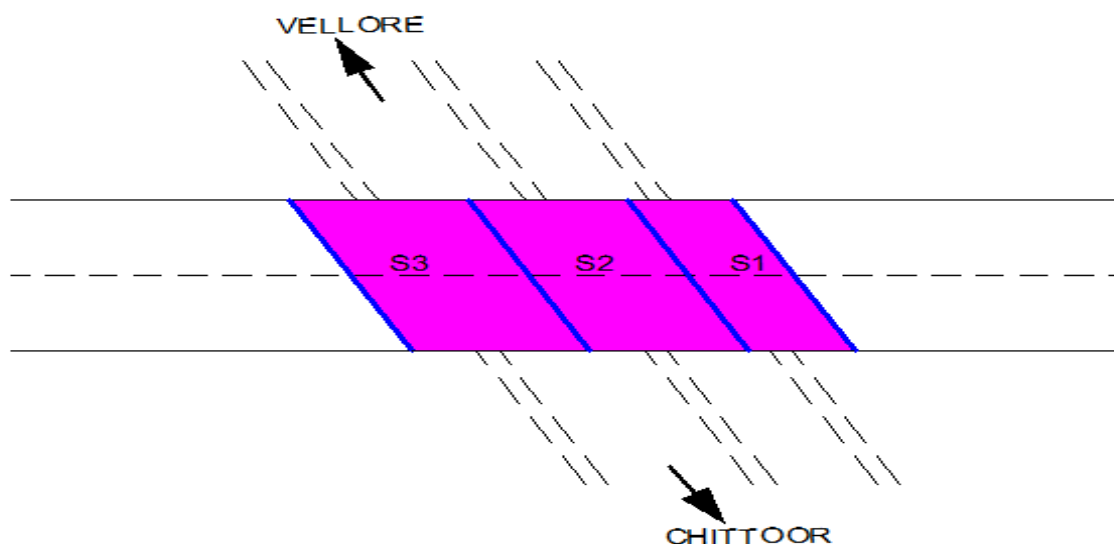
Summary:

- a) Cracks, Spalling has been observed in Girders, Piers & Slab.
- b) Reinforcement exposure has been observed in Piers & Slab.
- c) Severe cracks observed on the columns of Pier and Pier Caps.
- d) Leaching observed in Overhang portions of the Slab & Girders.
- e) Debris accumulated at Bearings
- f) Patch works have been observed.

PART 1.1 GENERAL INFORMATION AND OBSERVATION

a.	Name of the Bridge	ROB 561A
b.	Location Location co-ordinates	Near Chittoor Railway station (32/3-4) 13°13'00.3"N 79°06'12.3"E
c.	Type of Bridge	RCC Girder Bridge
d.	Year of Construction	Data Not Available
e.	Grade of Concrete (Slab)	Data Not Available
f.	Design Load	Data Not Available
g.	Carriageway Width	7.50 m
h.	Overall Deck Width	10.50 m
i.	Average Skew	20°
j.	Span arrangement (no x Span in meters)	1 x 7.7 + 2 x 10.75 (Railways Concerned)
k.	No of Railway Lines Crossings	03 tracks
l.	Vertical Clearance (from rail top to girder bottom)	5.268 m
m.	History of Repair/ maintenance	Yes, Patch works observed
n.	Existing structural drawings	Not available

Key Plan



PART 2 STRUCTURAL SYSTEM OF THE BRIDGE			
			N
a.	Foundation details		Not visible at site
	Type of foundation		
	Foundation Details (if available...)		
	Foundation Status (Check Settlement, abnormal Scour and Tilting, if any etc....)		
	Type of Damage (Check cracking, disintegration, decay, erosion, Cavitation's etc.,)		
b.	Abutment (A1 & A2)		
	Type	-NA-	
	Maximum height of Abutment from Ground level	-NA-	
	Abutment width along the Track	-NA-	
	Abutment thickness	-NA-	
	Condition (Crack, Settlement, Scour, Tilting, Steel Corrosion, Strains and other damages etc...)	-NA-	
	Efficiency of drainage of backfill Behind Abutments (Check functioning of weep holes, evidence of moisture on Abutment faces, etc.)	-NA-	
c.	Pier	04 Nos.	
	Type (Shape)	Portal Type	Portal Piers with 3 rectangular columns are Provided.
	Material	RCC	
	No. of Piers in a row/column	-	
	Maximum Height of Pier	4.6 m	
	Pier Diameter/Dimension		Cracks, Spalling, Reinforcement exposure and Leaching are observed at Pier caps & columns. ... (Refer Fig:4, 5, 6 &
	Pier Cap Material	RCC	
	Pier Cap Condition	Distresses Found	
	Pier Condition (Crack, Settlement, Scour, Tilting, Steel Corrosion, Strains and other Damages etc...)	Distresses Found	

d.	Bearing & Pedestal		
	Type of Bearings	Metal plate	
	General condition (Check Corrosion, cleanliness, seizing of plates silting, accumulations of dirt in case of submersible bridges)		Debris accumulated at bearings
	Type/ shape of Pedestal		
	General condition (Check Corrosion, cleanliness, seizing of plates silting)		There is no Pedestal
e.	Super Structure (Concrete Girder)		
	Type/Shape of Girder (I-beam, box-	Span-1 : Slab Span-2 & 3 : T-Girder	
	Material of Girder	RCC	
	No of Girder	05 Nos.	
	Dimensions of Girder		Girder Depth = 1.0 m Width of Girder = 0.6 m
	Structural System of Girder	Simply Supported	
	Condition of Girder	Distresses Found	Honey combing, Cracks & Spalling has been observed. (Refer Fig:10, 12 & 13)
	Type/Shape of Secondary Girder	Rectangular	
	Material of Secondary Girder	RCC	
	No of Secondary Girders	5 Nos.	
	Condition of Secondary Girder	Distresses Found	Honey combing, Cracks, Spalling has been observed.
f.	Slab		
	Material of Slab	RCC	
	Thickness of Slab	0.5 m	
	Condition of Slab from Bottom	Distresses Found	Honey combing, Cracks, Spalling, Reinforcement exposure has been
	Condition of Slab from Top		Not visible, as covered with wearing
	Condition of Flooring/Wearing Coat	Fair	(Refer Fig-3)
	Parapet wall/Railing		
	Type of Parapet wall/Railing	RCC	
	Height	1.0 m	Cracks have been observed.
	Condition of Wall/Railing	Distresses Found	Cracks, Spalling, Reinforcement exposure are observed.

PART 1.3 SURVEY OF SIGNS OF DISTRESS, DEFORMATION IN BRIDGES			
	Defects	Location	Notes
	Cracking in RCC components	Girders, Piers & Slab	Cracks observed in Girders, Piers & Slab
	Water seepage		No Water Seepage is observed
	Spalling	Girders, Piers & Slab	Spalling has been observed in Girders, Piers & Slab
	Reinforcement Exposure	Girders, Piers & Slab	Reinforcement exposure has been observed at Girders, Piers & Slab



Fig: 1 View of ROB



Fig 2: Bottom view of the Bridge



Fig 3: Top View of the bridge / Condition of wearing course



Fig 4: Cracks, Spalling & Reinforcement exposure observed in Pier-1



Fig 5: Cracks, Vegetation & leaching observed on Pier-1



Fig 6: Cracks, Spalling & Reinforcement exposure observed in Pier-2



Fig 7: Cracks, Spalling & Reinforcement exposure observed in Pier-2



Fig 8: Leaching at overhang portion of Slab of Span-2



Fig 9: Cracks at overhang portion of slab and spalling at Pier-3 column



Fig 10: Vegetation at Expansion joint & Spalling at Girder and Slab of Span-3



Fig 11: Debris accumulated at Bearings

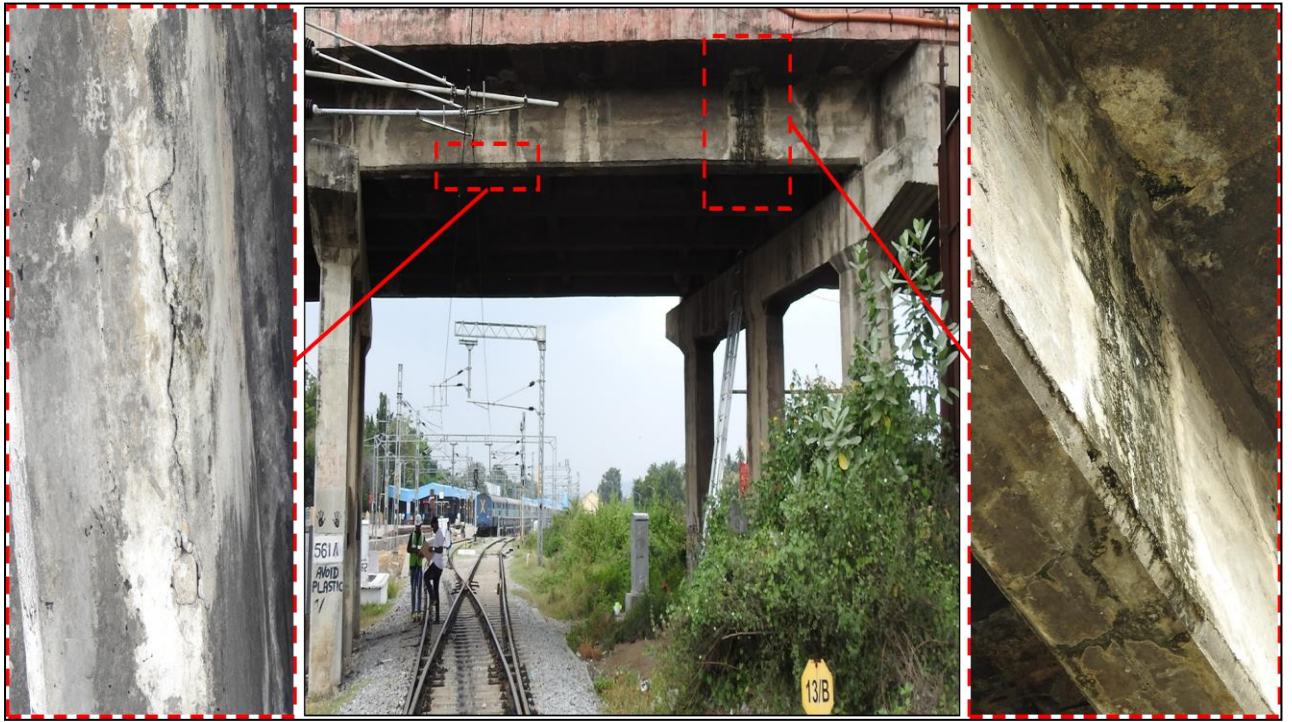


Fig 12: Spalling & Seepage observed at Exterior girder of Span-3



Fig 13: Spalling, Vegetation & Reinforcement exposure at Girder & Slab

2. NON-DESTRUCTIVE TESTING

The following tests were carried out to investigate strength and quality of “**Chittoor ROB (561A)**” elements such as piers, girder and slab as specified:

- Rebound Hammer test by Schmidt Rebound Hammer as per IS: 13311 part 2
- Ultrasonic Pulse velocity test, as per IS:516 part 5
- Crack width and depth
- Profometer Test
- Half-cell Potential Test

A. RESULTS AND OBSERVATIONS OF REBOUND HAMMER TEST:

The test results of rebound hammer test carried out on Girders, Piers, and Slab are presented in the Table1.

Table 1: Shows Observations Rebound hammer test

Sl. No	Member	Location ID	Method	Rebound Numbers	Corrected Rebound Number	Estimated Compressive Strength (N/mm ²)
1	Span-3 Pier-3, C1	1.8m from Ground Level	Horizontal	38,56,52.5,56, 47.5,48.5	49.8	42
2	Span-3 Pier-3, C1	2m from Ground Level	Horizontal	42,36.5,28,33, 46,32.5,	36.3	26
3	Span-3 Pier-3, C2	2m from Ground Level	Horizontal	54.5,37,35,47, 45,44.5	43.8	34
4	Pier-2, C2	1.8m from Ground level	Horizontal	40.5,56,47,49, 52,59	50.6	44
5	Pier-2, C3	1.6m from Ground level	Horizontal	61,62,63,63,55 ,56.5	60.1	61
6	@ Beam b/w C2 to C3 Centre	0.6m from C2	Horizontal	37,36,49,42,47 .5,42	42.3	32
7	Span-1 @pier cap	0.6m from bottom Girder	Horizontal	41,47,36,46,36 ,43	41.5	32
8	Span-1 @Slab	0.6m from A1	Horizontal	66.5,74,65,66, 59,55	64.3	71
9	Span-2 Girder -G5	0.6m from A1	Horizontal	42,46,37,47,41 ,40	42.2	32
10	Span-2 @Slab	@Slab in b/w G5-G4	Horizontal	37,47,39,45,51 ,56	45.8	37
11	Span-2 Girder -3	0.5m from A1	Horizontal	49,56,46,54,50 ,53.5	51.4	45
12	Span-2 Cross Girder	In b/w G3-G2	Horizontal	23,36,50,47,41 ,38	39.2	29
13	Span-2 Slab	In b/w G3-G2	Horizontal	47,43.5,55,44, 59.5,39	48.0	40
14	Span-3 Cross Girder	In b/w G4-G5	Horizontal	40,36,38,39,42 ,45	40.0	30
15	Span-3 Girder-5	0.6m from A2s	Horizontal	65,64,54,59,44 ,64	58.3	58

Note: Estimated compressive strength is referred from the standard charts developed for particular strength. Probable accuracy is $\pm 25\%$

B. RESULTS AND OBSERVATIONS OF ULTRASONIC PULSE VELOCITY TEST (UPV Test):

The test results of UPV test carried out on Main girders, cross girder, Pier, and Slab are presented in the Table2.

Table 2: Shows Observations of Ultrasonic Pulse Velocity test

Sl. No .	Member Id*	Location	Test Method	Path Thickness (mm)	Pulse Velocity (km/sec)	Corrected Pulse Velocity (km/sec)	Concrete Quality As per Referred Table - II
1	Span-3 Pier-3, C1	1.8m from Ground Level	In-Direct	400	1.873	1.873	Poor
2	Span-3 Pier-3, C1	1.8m from Ground Level	Direct	600	3.872	3.872	Good
3	Span-3 Pier-3, C2	2m from Ground Level	In-Direct	400	2.503	2.503	Poor
4	Pier-2, C2	1.8m from Ground level	In-Direct	400	2.162	2.162	Poor
5	Pier-2, C3	1.6m from Ground level	In-Direct	400	4.338	4.838	Excellent
6	@ Beam b/w C2 to C3 Centre	0.6m from C2	In-Direct	400	1.802	1.802	Poor
7	Span-1 @pier cap	0.6m from bottom Girder	In-Direct	400	2.717	2.717	Poor
8	Span-1 @Slab	Near p2	In-Direct	400	2.139	2.139	Poor
9	Span-2 Girder -G5	0.6m from A1	In-Direct	400	2.170	2.170	Poor
10	Span-2 @Slab	@Slab in b/w G5-G4	In-Direct	400	2.049	2.049	Poor
11	Span-2 Girder -3	0.5m from A1	In-Direct	400	3.945	4.445	Excellent
12	Span-2 Cross Girder	In b/w G3-G2	In-Direct	400	1.496	1.496	Poor
13	Span-2 Slab	In b/w G3-G2	In-Direct	400	2.155	2.155	Poor
14	Span-3 Cross Girder	In b/w G4-G5	In-Direct	400	4.396	4.896	Excellent
15	Span-3 Girder-5	0.6m from A2s	In-Direct	400	3.053	3.553	Good

CONCRETE QUALITY GRADE CHART FOR ULTRASONIC PULSE VELOCITY TEST (As per IS: 516 Part 5, Clause 2.5.2, Table -I)

PULSE VELOCITY(Km/Sec)	CONCRETE QUALITY
Above 4.40	Excellent
3.75 to 4.40	Good
3.0 to 3.75	Doubtful
Below 3.0	Poor

Note:

1. In the case of "doubtful" quality, it may be necessary to carry out the further test.
2. As per IS: 516 (Part 5) Clause 2.4.3.2.5, Surface probing, in general, gives lower pulse velocity than in case of cross-probing and depending on a number of parameters, the difference could be of the order of about 0.5 km/sec.

C. OBSERVATIONS OF CRACK WIDTH AND DEPTH:

Table 3: Shows Observations of Crack width and depth

Sl. No	Member	Location ID	Width (mm)	Depth (m)	Remarks
1	Pier-3, C-2	1.8m from bottom	0.60	-	Throughout Crack
2	Pier-1, C-3	0.8m from bottom	0.60	0.041	

C. OBSEVATIONS OF PROFOMETER TEST

Table 4: Shows Observations of Profometer Test

S. No.	Member	Location ID	Avg. Cover(mm)
1	Column-2	1.2m from GL	42.5
2	Column-2	2m from GL	54.6

E. OBSEVATIONS OF HALF CELL POTENTIAL TEST

Table 5: Shows Observations of Half-cell potential test

S. No.	Member	Location ID	Measured Potential (mV)	Avg. Potential Value(mV)	Probability of Corrosion
1	Span-1 @pier cap	Pier Cap-1	-169, -127, -83, -191, -206, -309, -177	-180	Low (10% of risk corrosion)
2	Span-1 @pier cap	Pier Cap-1	-212, -332, -141, -261, -481	-285	Intermediate Corrosion risk
3	Span-3 @Slab	Slab in b/w G4-G5	-274, -295, -293, -249, -309, -222, -255	-271	Intermediate Corrosion risk

Half Cell Potential Test Chart of Uncoated Reinforcing steel in Concrete

(Criteria of corrosion according to ASTM C876-91)

Average Potential Value (mV)	Corrosion Condition
<-500	Severe Corrosion
<-350	High (90% of risk corrosion)
-350 to -200	Intermediate corrosion risk
>-200	Low (10% of risk corrosion)

NDT Summary:

- Estimated strength as per rebound hammer test indicated that concrete strength is >26MPa.
- Ultrasonic Pulse Velocity test indicates that the quality of concrete varies from Poor to Excellent.
- Half-cell potential test shows that the Rebar is in Intermediate risk of Corrosion.
- Crack width test shows that the observed cracks at Pier columns are major structural cracks (>0.3mm)



Fig 15: UPV test at site



Fig 16: Rebound hammer test at site



Fig 17: Half-cell potential test at site

3. Repair and Retrofitting Recommendations

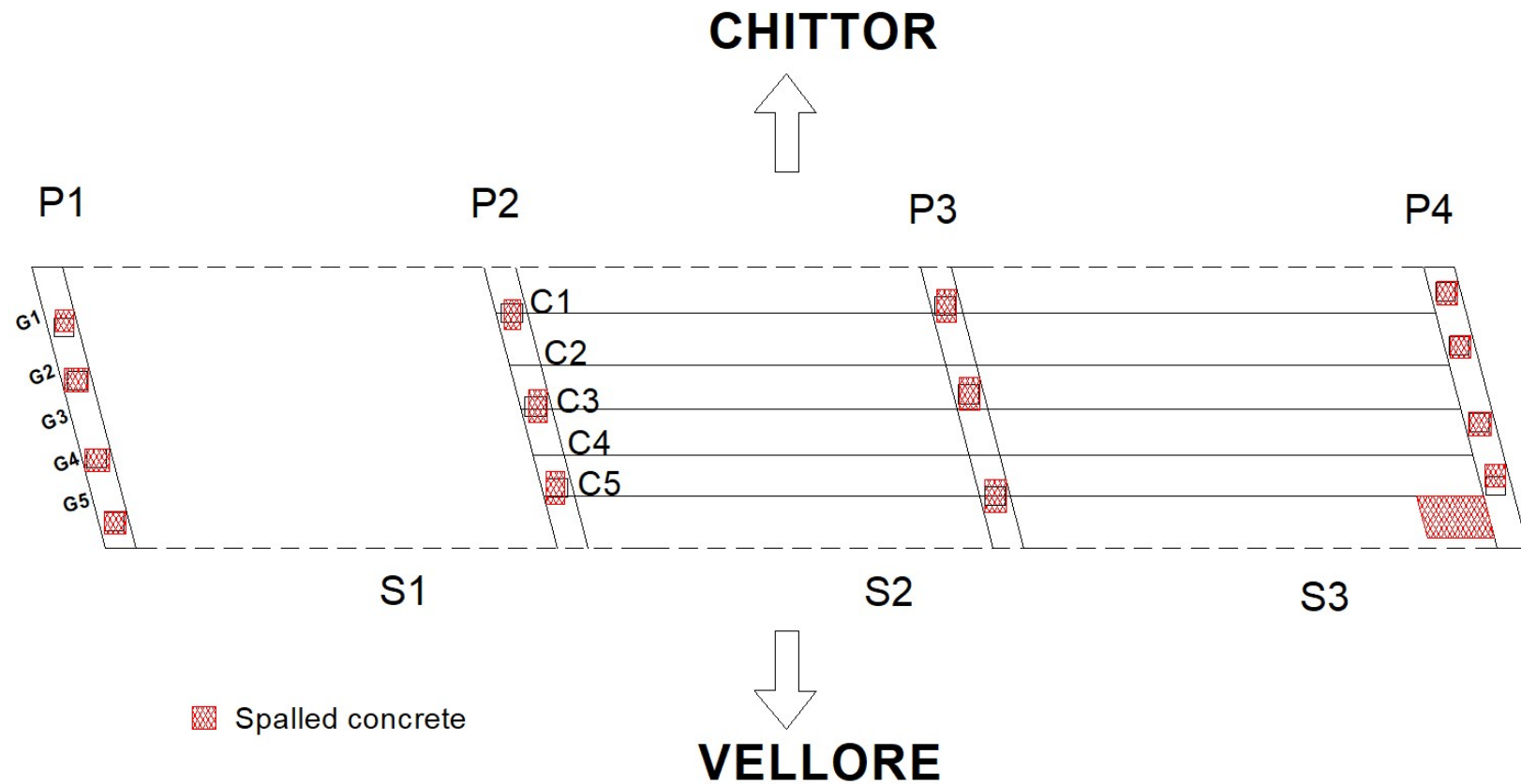


Fig: 18 Bottom view of ROB with damaged locations

The detailed visual observation of ROB, the following observations are made. Major Spalling and Reinforcement exposure are observed at Deck slab and piers as shown in the above figure, Corrosion is seen in the exposed rebars of the Slab, Major Structural cracks are observed at almost all the columns of the pier.



Fig: 19 Major Cracks at Columns and Reinforcement exposure.

ANNEXURE

Annexure-1a: Repair methods

Defects/Deficiency	Debris accumulation
Remedial Measures	Cleaning

Work sequence

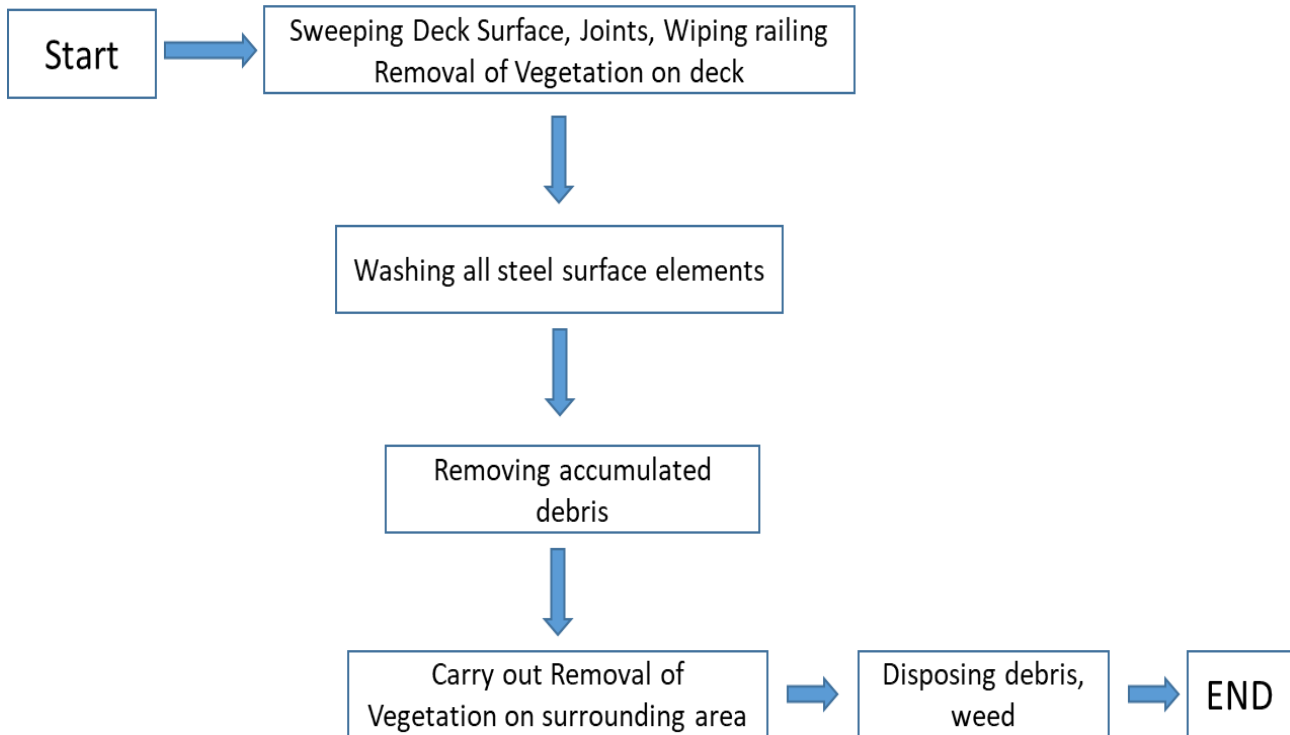


Figure:1.1 Work sequence of removal of vegetation and accumulated debris

REQUIREMENT, SPECIFICATION

Material Requirement

Water to be used for cleaning of the bridge components shall be clean and free from unwanted foreign materials such as sediments, salt contaminants, chemicals, grease, oil, rubbish, and other substances which are harmful to the bridge components.

The contractor shall obtain necessary approvals of the source of water to be used for cleaning. Engineer's approval shall be taken on the source and quality of water. All necessary tests shall be performed on water samples at laboratories to be specified by the Engineer, and test certificates shall be provided as required.

Work Requirement

1. General

All accumulated foreign materials shall be removed from bridge sidewalks, bridge decks, top of curbs, beam flanges, gusset plates, abutment bridge seats, and other locations specified and as directed by the Engineer, prior to cleaning with water pressure. Removal shall be performed using hand brooms, hand shovels, scrapers, vacuum cleaners, or other methods acceptable to the Engineer. The removed materials shall be collected and disposed at an approved waste area in

accordance with governing local regulations. At no time shall these materials be allowed to be disposed into the river or on dry land portions below the bridge.

2. High pressure water

Salt contaminants, dirt, and other detrimental foreign matters shall be removed without damaging the structure. If high-pressure water is used, the maximum water pressure shall not be so high that may cause damage. The cleaning operation shall be discontinued if the foreign materials have not been easily removed or if cleaning operation is causing damage to existing structure.

All deck drains and its accessories shall be flushed with high-pressure water after the accumulated foreign material has been carefully removed. Drain systems may have to be disassembled to remove large blockages of accumulated foreign material. Should this be necessary, these shall be returned to their original configuration immediately after cleaning. Drainage systems shall drain properly after cleaning.

Annexure-1b: Repair methods

Defects/Deficiency	Spalling, Honeycomb
Remedial Measures	Patching

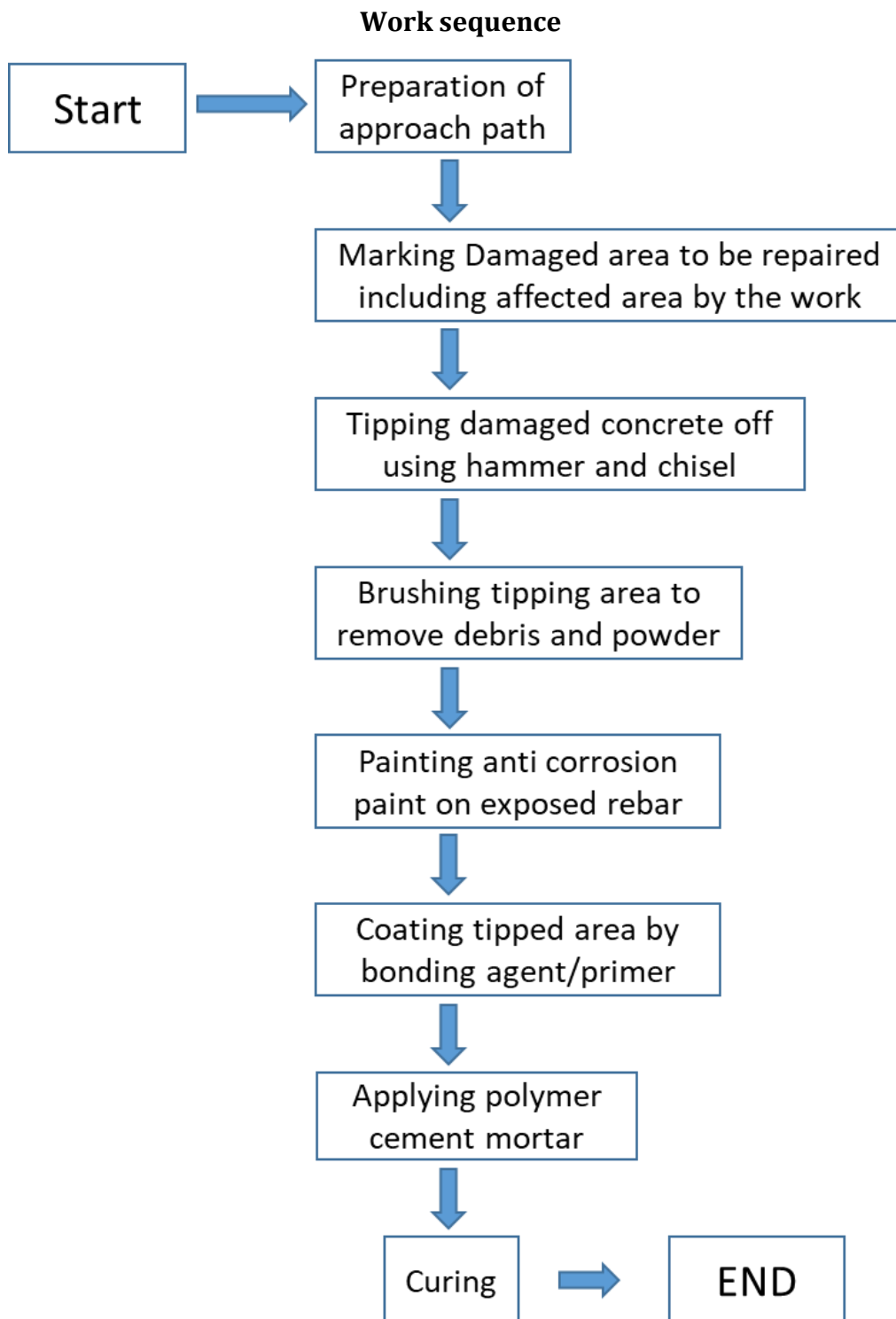


Figure: 1.2 Work sequence of Epoxy Coating

Required Equipment/Tool and Material

Material List –

- Polymer Cement Mortar
- Portland Cement
- Epoxy Bonding
- Zinc rich primer

Tool/Equipment List –

- Chisel
- Portable Generator
- Wire Brush

Requirement, Specification

Material Specifications

The Engineer through mill certificate of the supplier shall approve the material.

1) Polymer Cement

Polymer cement mortar shall conform to the requirements of the specifications shown in Table 1.1 or equivalent ASTM Specifications.

Table:1.1 Specification of Polymer Cement for Patching

Property	Test Method	Specification
Initial setting time	-	above 60 minutes
Shrinkage	ASTM D2566	below 0.05 %
Thermal expansion	ASTM C531	2.0×10^{-5} mm/mm/ ° C
Slant shear bond to concrete	ASTM C882	Concrete failure above 15 N/mm ²
Compressive strength	ASTM D695M	above 20 N/mm ²

2) Epoxy bonding agent

The epoxy-bonding agent to concrete surface shall conform to the requirements of the specification indicated in Table 1.2 (Anti-corrosion zinc rich primer shall be applied to exposed rebar).

Table 1.2 Specification of Epoxy Bonding Agent to Concrete Surface

Property	Test Method	Specification
Compressive strength	ASTM D695M	70 N/mm ²
Flexural strength	ASTM D790M	40 N/mm ²
Tensile strength	ASTM D638M	30 N/mm ²
Tensile shear bond to steel	ASTM 1002	15 N/mm ²
Slant shear bond to mortar	ASTM C882	15 N/mm ²
Bond Strength of Cured Concrete to Fresh Concrete	ASTM D7274	15 N/mm ²

- 3) Zinc Rich Primer The zinc rich primer to rebar shall be in accordance with the requirements of the specifications in Table 1.3

Table 1.3 Specification of Zinc Rich Primer for Rebar

Property	Test Method	Units	Specification
Gloss @ 60°Angle	ASTM D 523	-	Flat
Adhesion	ASTM D 3359	-	Minimum 3A
Salt Spray Resistance	ASTM D3-37	-	Excellent
%Zinc by Weight in Dried Film Test		%	87.5±2

Work requirement

1) General

Patching repair works using Portland cement mortar shall be carried out in accordance with provisions of relevant standards and manufacturer's specifications. Patching, as a minor repair work, should be carried out using an appropriate means to access the repair area before the defect worsens. Patching repair work method using polymer cement mortar shall be carried out as follows:

2) Marking of patching area

Evaluate surface area to be patched using hammer for hollow sounding delaminated area (or by using instruments to detect unsound concrete).

Mark the area to be patch-repaired with paint or marker. Ensure complete coverage of the damage.

- 3) Tipping damaged concrete off Using a small sledgehammer and chisel, remove all damaged concrete at the edges and corners of area to be repaired. Use a wire brush to remove loose debris. Care should be taken to ensure that no reinforcement is damaged.

Surface shall be cut to expose the reinforcement and to reach the sound concrete substrate, without breaking the concrete behind the reinforcement. If rebar is exposed, anticorrosion agent coating should be applied on the bar surface prior to patching. All works shall be subjected to the approval of the designated Engineer.

Patch areas that are within 600 mm of each other should be combined into a single large patch. If necessary, provide formwork around the damaged concrete to straighten the edges of the damaged section.

- 4) Coating tipping area Concrete surfaces to receive repair mortar shall be prepared by mechanical scrubbing to remove loose materials, surface laitance, organic contaminants and moss. The clean and dust free surface shall then be coated by a bonding primer.

Care shall be taken to ensure that vibration associated with the repair works does not cause delamination of existing adjacent plaster or concrete.

5) Applying bonding agent and anti-corrosion paint

Apply bonding agent to the damaged area for the patch material to adhere. Additionally, concrete nails/bids may be set to reinforce the repair.

If rebar is exposed, anticorrosion coating should be applied on the bar surface prior to patching.

6) Patching

mortal Prepare the mortar mix in a bucket using equipment approved by the Engineer. Use a trowel to spread fresh mortar over the area, covering the concrete nails driven halfway in the old concrete. Smoothen and level the mortar with a trowel. Polymer cement mortar is suitable for both vertical and horizontal surface applications, with a thin coating of up to 15 mm.

7) Finishing

As may be required, the mortar surface can be smoothed using a trowel or broom finished. The texture of the finish of the final repair mortar layer shall match the finish of the existing surface. The repair mortar application shall be built up to the original surface profile in layers not exceeding 20 mm with the final layer not exceeding 15 mm, unless otherwise recommended by the manufacturer and approved by the Engineer.

The Engineer may approve repair mortar application thickness of up to 50 mm for lightweight mortars, provided the mortar manufacturer furnishes technical data to justify a layer thickness of greater than 20 mm.

8) Curing All types of concrete repair with repair mortar need thorough and continuous curing to develop strength and impermeability. Curing also minimizes drying shrinkage while bond strength is developing. Curing of the repair mortar shall be in accordance with the manufacturer's instructions related to the polymer modified additive. Where curing agents are specified by the manufacturer, they shall be applied immediately after the surfaces have been scarified for the next repair mortar layer or troweled to a finish.

Annexure-1c : Repair methods

Defects/Deficiency	Reinforcement Exposure, Spalling and Cracking
Remedial Measures	Shotcrete

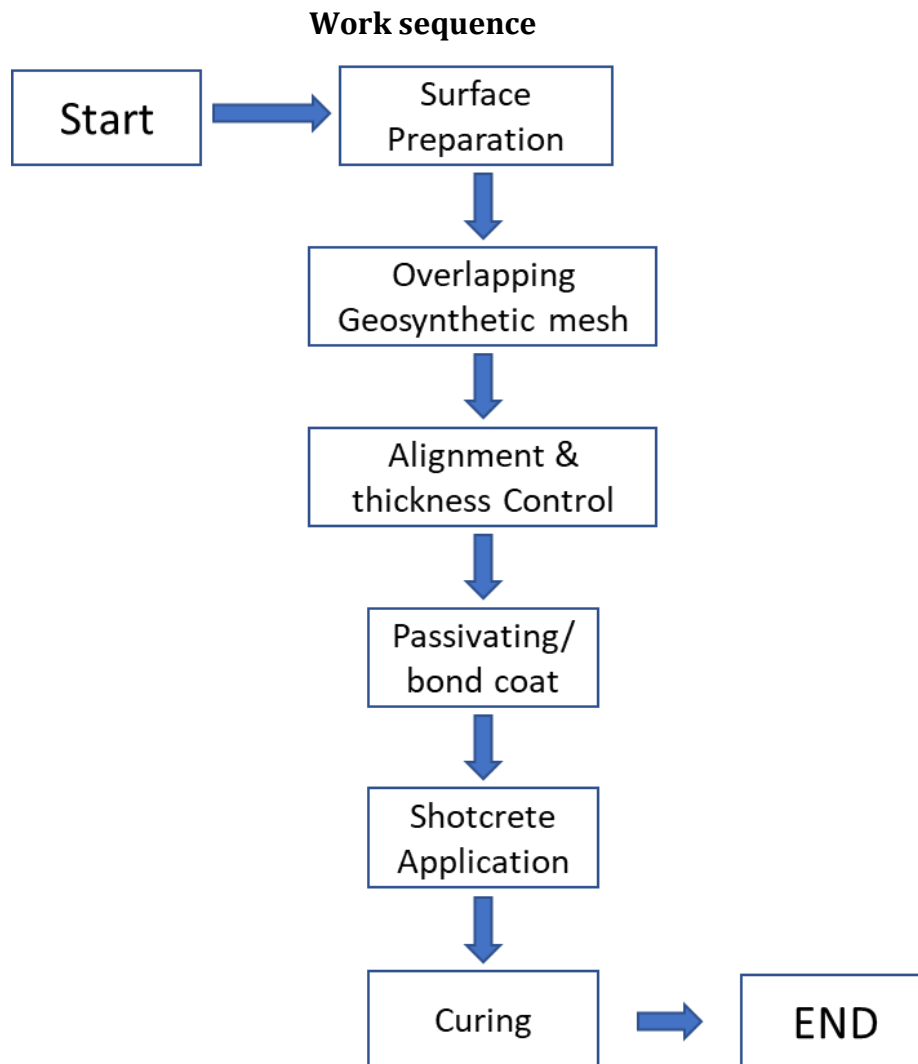


Figure: 1.4 Work sequence of Shotcrete

Required Equipment/Tool and Material

Materials

- Cement
- Fine Aggregate
- Coarse Aggregate
- Admixture
- Water

Tools/Equipment

- Batching and mixer
- Guniting equipment
- Compressor

Requirement, Specification

Materials

Cement: The cement shall be ordinary Portland cement of 43 grade conforming to relevant B.I.S. Code of Practice

Aggregates:

Sand for shotcrete shall comply with the requirements given in IS 383 and graded evenly from fine to coarse as per Zone-II and Zone III grading. Sand failing to satisfy this grading may, however, be used only if pre-construction testing establishes that it gives good results.

Coarse aggregate when used shall comply with the requirements of IS 383. It shall, generally conform to the grading given below:

Table 1.5 Gradation of the coarse aggregate

GRADING OF COARSE AGGREGATES	
IS Sieve Designation, mm	Percentage Passing by mass for aggregate of nominal maximum size
10	100
4.75	10 – 30
2.36	0 – 10
1.18	0 - 5

All over sized pieces of aggregate shall be rejected by screening. Gradation of the combined coarse and fine aggregate mixture used for shotcrete shall generally lie between the following limits.

Table 1.6 Gradation of the combined coarse and fine aggregate mixture

I.S Sieve	Percent passing by Weight Gradation
10 mm	100
4.75 mm	72 - 85
2.36 mm	52 - 73
1.18 mm	36 - 55
600 microns	28 - 38
300 microns	7 - 20
150 microns	0 - 8

Water used for shotcrete shall conform to the requirement of I.S. 456-2000

Admixture: Guniting admixtures & quick setting agents may be used to minimize the rebound loss and increase the bond & enable thicker layers per coat. The admixture shall meet the requirement of I.S. 456 and I.S. 9103

Concrete: The grade of concrete shall be as specified i.e. the characteristic compressive strength of 15 cm cube at 28 days should be as specified. The water cement ratio for shotcrete shall be within the range 0.45- 0.50 by mass.

Work requirement

- 1) This specification covers only to the extent of restoring the concrete slab by way of short creating. The specification excludes bearing replacement, expansion joint treatment etc.
- 2) Remove all loose and spalling concrete and expose the corroded reinforcement wherever possible by careful chipping.
- 3) Carryout thorough sand blasting using dry coarse sand or by grinding.
- 4) Fix 8 mm dia PVC nozzles at all honey combed areas and wherever cavities exist. At all other vulnerable areas drill 12/14 mm dia hole to a depth of 50 to 75 mm and fix 8 mm PVC nozzles.
- 5) Now drill 12/14 mm dia holes at a spacing of 500 mm c/c to a depth of 50 mm and fix 8 mm dia shear connectors using polyester resin grout.
- 6) Fix 50 mm x 50 mm x 4 mm Geo grid tied to the exposed reinforcement / shear connectors.
- 7) Carryout 40 to 50 mm thick short - creating to underside of slab etc., using a mix proportion of 1: 2: 2 (cement: sand: aggregate).
- 8) Now carryout cement injection grouting through the nozzles in sequential manner until rejection.
- 9) Carryout curing for 7 days.
- 10) Note: Grouting is done only after shotcrete to fill any leftover voids in the concrete slab.

Summary

- Visual inspection of Chittor ROB (561 A) has been conducted on 27th Nov 2019, following observations were made
 - a) Cracks, Spalling has been observed in Girders, Piers & Slab.
 - b) Reinforcement exposure has been observed in Piers & Slab.
 - c) Severe cracks observed on the columns of Pier and Pier Caps.
 - d) Leaching observed in Overhang portions of the Slab & Girders.
 - e) Debris accumulated at Bearings
 - f) Patch works have been observed.
- Non-Destructive Testing of Chittor ROB (561A) has been conducted on 03rd Sept 2020, following observations were made
 - a) Estimated strength as per rebound hammer test indicated that concrete strength is >26MPa.
 - b) Ultrasonic Pulse Velocity test indicates that the quality of concrete varies from Poor to Excellent.
 - c) Half-cell potential test shows that the Rebar is in Intermediate risk of Corrosion.
 - d) Crack width test shows that the observed cracks at Pier columns are major structural cracks (>0.3mm)
- Based on visual inspections and NDT Testing and its interpretations following recommendations were made:
 - a) Epoxy Grouting of cracks observed at girders, pier columns and slab.
 - b) Jacketing of the severely cracked columns should be done
 - c) Application of Anti corrosive paint for exposed rebars with Minimal deterioration
 - d) Replacement of severely Corroded bars in slab and Shotcreting of the deteriorated concrete
 - e) Clearing of accumulated debris.

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